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Experimental study of mooring system and pile system on floating breakwater reviewed from the transmission coefficient

 Hotma Harapan Saragih^{1*},  Roesdiman Soegiarso²,  Indra Noer Hamdhan³

^{1,2,3}Universitas Tarumanagara, Jakarta, Indonesia.

Corresponding author: Hotma Harapan Saragih (Email: saragih.hotma@gmail.com)

Abstract

The main problem in the Indonesian Ocean, especially along the west coast of Sumatra to the south coast of Java, is wave height. High sea waves can damage coastal areas. One method of mitigating this issue is the use of breakwaters, which function as wave energy dampers in specific areas. Large wave impacts can be reduced by decreasing the incoming wave energy so that waves approaching the coast have less energy. Therefore, a wave breaker construction is necessary to break and transmit wave energy effectively. This study aims to determine the transmission coefficient (K_t) value through physical model testing. The results from the physical model test indicated a transmission coefficient of 0.621 with the MU1A d50 L137 draft10 f0167 A010 test model /Regular Wave. The research findings suggest that a floating breakwater with a mooring system, placed at a depth of d50 with a draft of 10 cm (half submerged), can dampen wave energy when affected by regular waves, demonstrating its effectiveness in wave energy reduction.

Keywords: Floating breakwater, Mooring system, Physical model, Pile system, Transmission coefficient.

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1. Introduction

The main problem in the Indonesian Ocean, especially in the west coast of Sumatra to the south coast of Java, is the height of the waves. The height of the sea waves can damage the coastal areas. One method of overcoming the coast is the use of breakwaters, where the structure functions as a wave energy damper in certain areas. The onslaught of large waves can be dampened by reducing the energy of the incoming waves, so that the waves that head towards the coast have less energy. For this reason, a wave breaker construction is needed that functions to break and transmit wave energy. Various types of breakwaters have been known, namely rubble mound breakwater, caisson breakwater, composite breakwater, and floating breakwater [1].

Floating breakwater has advantages compared to other types of structures, namely that the structure can be used in soft ground conditions [2] and sea depths of more than 10feet [3]. *Floating breakwater* produces minimal interference to water circulation, sediment transport, and fish migration [4] effectively dampens waves less than 2 meters [5] this structure can also be easily moved and reassembled with a different layout to another location [3, 6, 7] does not cause *scouring* [2]. However, *floating breakwaters* also have disadvantages, including being less effective in reducing waves for short waves. Practically, the upper limit of the wave period is 6 seconds with a frequency of 1.6 radians/second; if the mooring structure fails, it will cause a disaster [4] and this structure requires high maintenance costs compared to conventional breakwaters [5].

A *floating breakwater* reduces wave energy by absorbing the wave through its interaction with the wave. Studies on wave transmission and reflection have been conducted by several researchers, both with physical and numerical models on various types of floating breakwaters with different wave characteristics, types of *breakwater*, and the geometric shape of the structure under review. Broadly speaking, there are four types of floating *breakwater*: *pontoon*, *tethered float*, *mat*, and *box* [2].

Study *floating breakwaters* usually focus on their performance, namely the ability of the structure to dampen waves (transmission coefficient), the stability of the structure and mooring system in various wave conditions, structure configuration, and location depth [8]. Research conducted by Ofuya [9] with three variations of pontoon draft (s/d), it was shown that wave transmission through the pontoon was greatly influenced by the height of the incoming wave (H_i), wavelength (L), width of the structure (B), depth of water in front of the structure (d), and pontoon draft (s). The transmission coefficient value will reach a minimum at $B/L = 0.55$ and relative depth (s/d) 0.27. Research by Priadi [10] on floating breakwater pontoon type shows that the pontoon draft(s) will have a significant influence on the transmission coefficient value if it exceeds one-third of the water depth. Dirgayusa and Yowono [11] studied wave transmission through horizontal pipe arrangements, and it was found that the longer the pipe used, the smaller the wave transmission. Additionally, the smaller the diameter of the pipe used, the smaller the wave transmission that occurs.

The research principles of Dirgayusa and Yowono [11] were further studied by Walukow [12] in a series of horizontal plates, it can be seen that the wave transmission that occurs in the floating breakwater plate series ranges from 29% ($B/L=0.90$) to 94% ($B/L=0.15$). The energy transmitted by this type of PGT is influenced by the incoming wave parameters (H, L), structure length (B), water depth (d), and distance between plates (s). The longer the plate used, the smaller the wave energy transmitted. The shallower the waters, the smaller the wave energy transmitted.

Heng [13] researched a *floating breakwater stepfloat* type with specifications of 80 cm long, 25 cm wide, 13 cm high, and piled with a pile system at a depth of 20 cm. Heng reported that this type of breakwater can reduce waves by 50-70%. Sujantoko et al. [14] studied the effects of mooring rope angle and water depth on its tension in a floating breakwater porous saw type. It was found that the greater the angle of the mooring rope, the greater the tension experienced by the mooring rope and the *floating breakwater*. This type provides a smaller mooring rope tension compared to the pontoon type. The dynamic behavior of *floating breakwater* types of porous saws has also been studied by Sujantoko Djatmiko et al. [15] it can be seen that the movement surge, *pitch*, and *yaw* are not affected by side waves, so the value of *Response Amplitude Operators* (RAO) is very small, almost approaching zero. Movement of the floating breakwater only affects movements of *heave* and *roll*.

The effectiveness of a wave retaining structure can be assessed by examining the amount of wave energy transmitted through it; the smaller the reduction in wave energy, the higher the wave transmission coefficient. The formulation of the wave transmission coefficient relationship model is derived by analyzing measurements of transmitted and incoming waves based on test variables, namely the height and period of incoming waves, the arrangement and type of the structure, the mooring system of the floating breakwater, and the pile system of the floating breakwater.

2. Method

2.1. Research Location

The research was conducted at the Wave Laboratory, Ocean Engineering, Faculty of Civil and Environmental Engineering, Bandung Institute of Technology, Indonesia.

2.2. Model Planning and Scale Floating Breakwater

Physical model design *floating breakwater* must be done as well as possible so that it can truly represent the characteristics of the actual prototype. Therefore, what must be done in designing a structural model is to determine the dimensional scale between the prototype and the physical model by referring to the laws of geometric similarity, dynamic similarity, and kinematic similarity. In addition, the weight scale is also considered because it will affect buoyancy (*buoyancy*). Based on this, a scale of 1:50 is obtained as shown in Table 1 and Table 2.

Table 1.
Prototype and Model Dimensions *Floating Breakwater*.

Dimension	Prototype		Model	
	1	2	1	2
Length (m)	55,00	55,00	1,10	1,10
Width (m)	30,00	15,00	0,60	0,30
Height (m)	10,00	10,00	0,20	0,20

Table 2.
Prototype and Model Parameter Scales *Floating Breakwater*.

Parameter	Prototype (m)	Model (m)
Wavelength (m)	10,00	0,20
	5,00	0,10
	2,50	0,05
Water Depth (m)	25,00	0,50
	40,00	0,80

The model *floating breakwater* in this test is designed using a steel plate as the basic material. The selection of the basic material of the model is based on considerations of strength, manufacturing process, and ease of obtaining. The printing of the model shape is done based on the scaling that has been made and adjusted to the laboratory dimensions.

Model *floating breakwater*, which has been designed, before the laboratory test is carried out, calibration is performed. Calibration is conducted to adjust the mass distribution to match the desired mass distribution. The parameters for determining the mass distribution include the location of the center of mass (*center of gravity*, CG) and the radius of gyration of the structure. Additionally, buoyancy (*buoyancy*) is reviewed to assess the total mass of the model. The model will be declared valid if it has been calibrated and has the same dimensions as the scaled structure. In this model, a tolerance value of 5% of the targeted value is used.

2.3. Configuration Floating Breakwater

The behavior of wave transmission and wave reflection is influenced by the arrangement of the floating *breakwater* to the direction of the wave. Therefore, in this study, the arrangement was varied to understand the extent of the influence of the arrangement of the floating *breakwater* on the efficiency of wave transmission and reflection. In this study, the arrangement of the placement of the floating *breakwater* is *planned*, as shown in Figures 1 and 2.

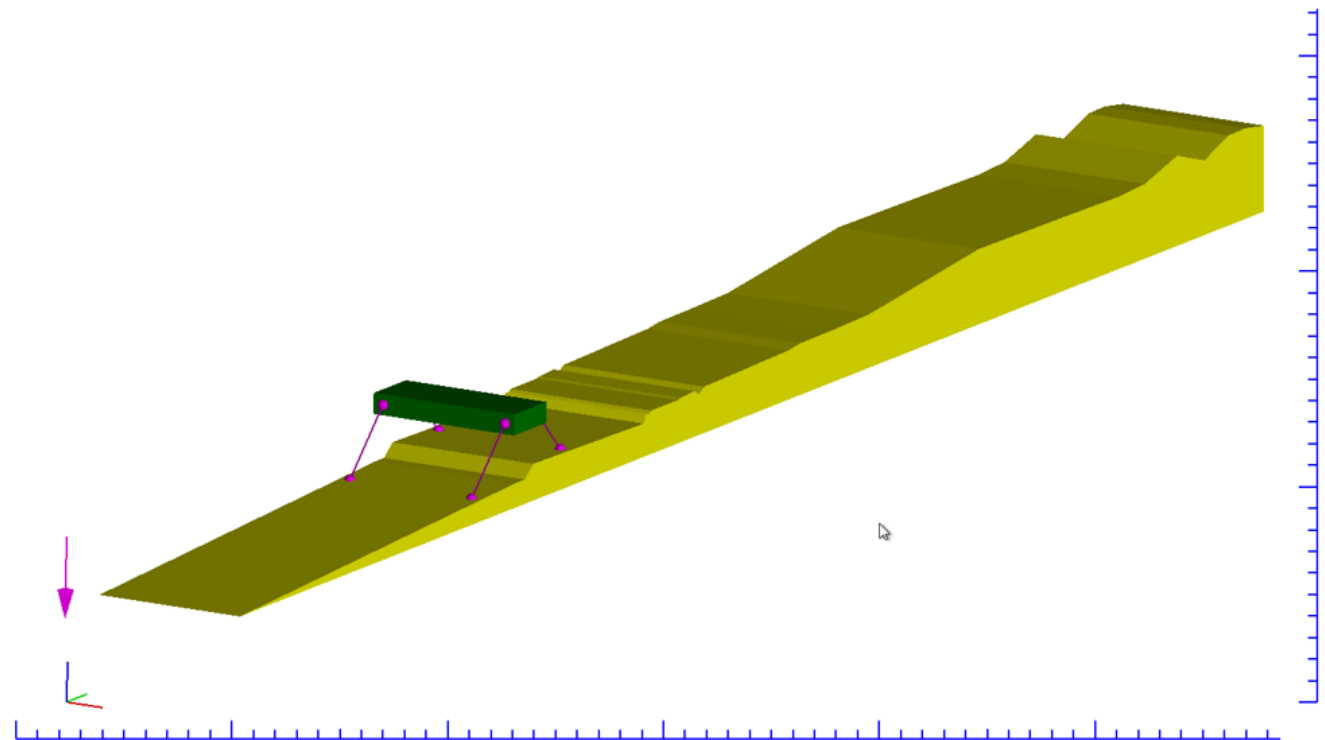


Figure 1.
Model Uji MU1A (110 x 60 x 20) *mooring system*.

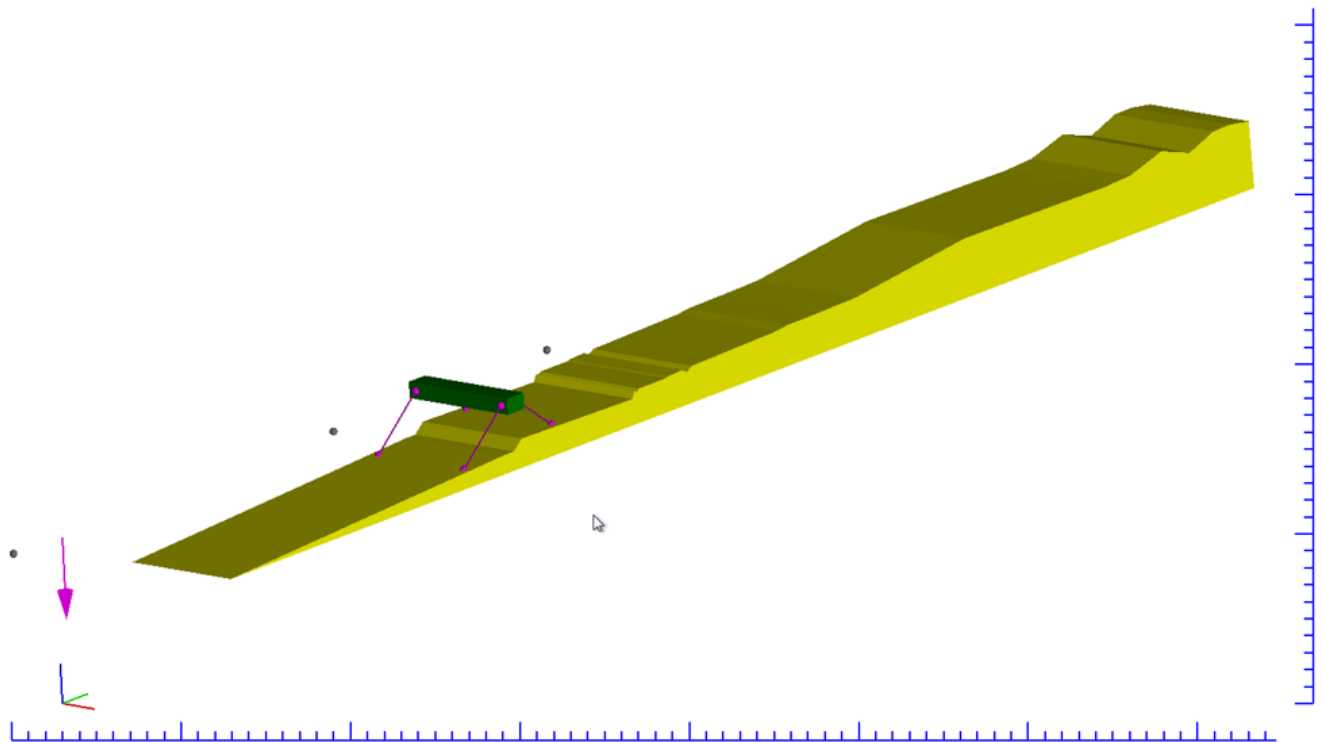


Figure 2.
Model Uji MU1B (110 x 30 x 20) mooring system.

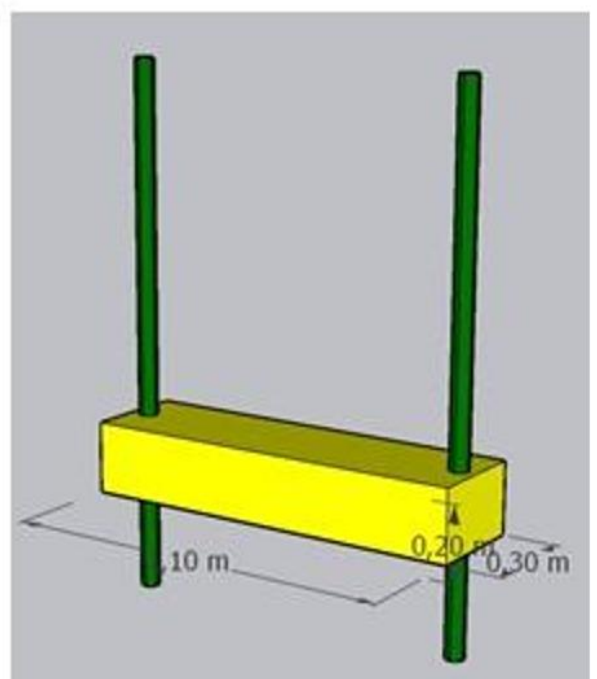
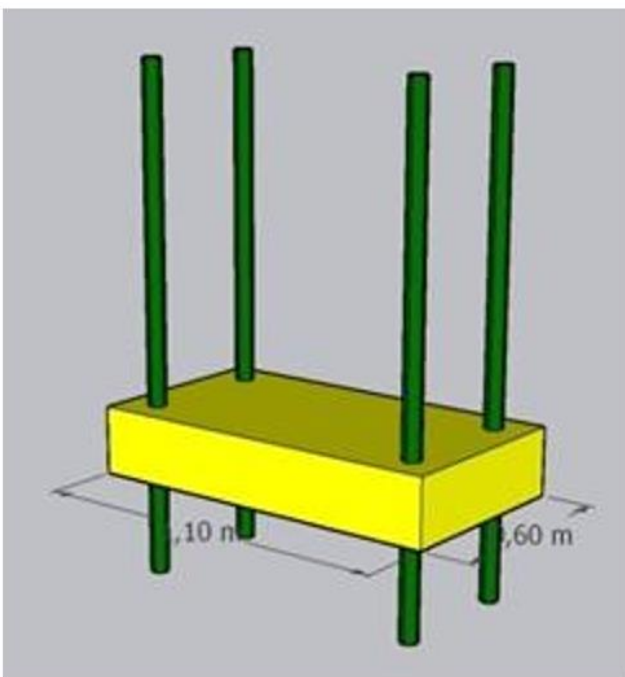


Figure 3.
Pile System on Test Models MU2A (110 x 60 x 20) and MU2B (110 x 30 x 20).

2.4. Wave Flume

The equipment used in this research is a Wave *Flume* (wave test pool) with a length of 40 m, a width of 1.20 m, and a height of 1.50 m, equipped with a plunger-type wave generator. The waves generated can be regular or irregular. The maximum wave height that can be generated is 0.2 m, with a wave period ranging from 4.0 to 8.0 seconds for regular waves. The irregular wave spectrum that can be generated includes Jonswap, Pierson-Moskowitz, ISSC, and ITTC, with a maximum wave height and period of 0.2 m and 8 seconds.

2.5. Wave Probe

Wave *Probe* is a wave height measuring tool. When the tool is immersed in water, the electrode measures the conductivity of the water volume. The conductivity varies proportionally with changes in water level elevation. In this study, four wave probes were installed (Figure 3). The wave type used in this experiment is the Water Tide Meter WTM-

902, made of stainless steel 304 and PTFE string AWG 30. The maximum sensor capacity is 800 mm, the resolution is 1 mm, and it has the ability to record 60 data points per second.

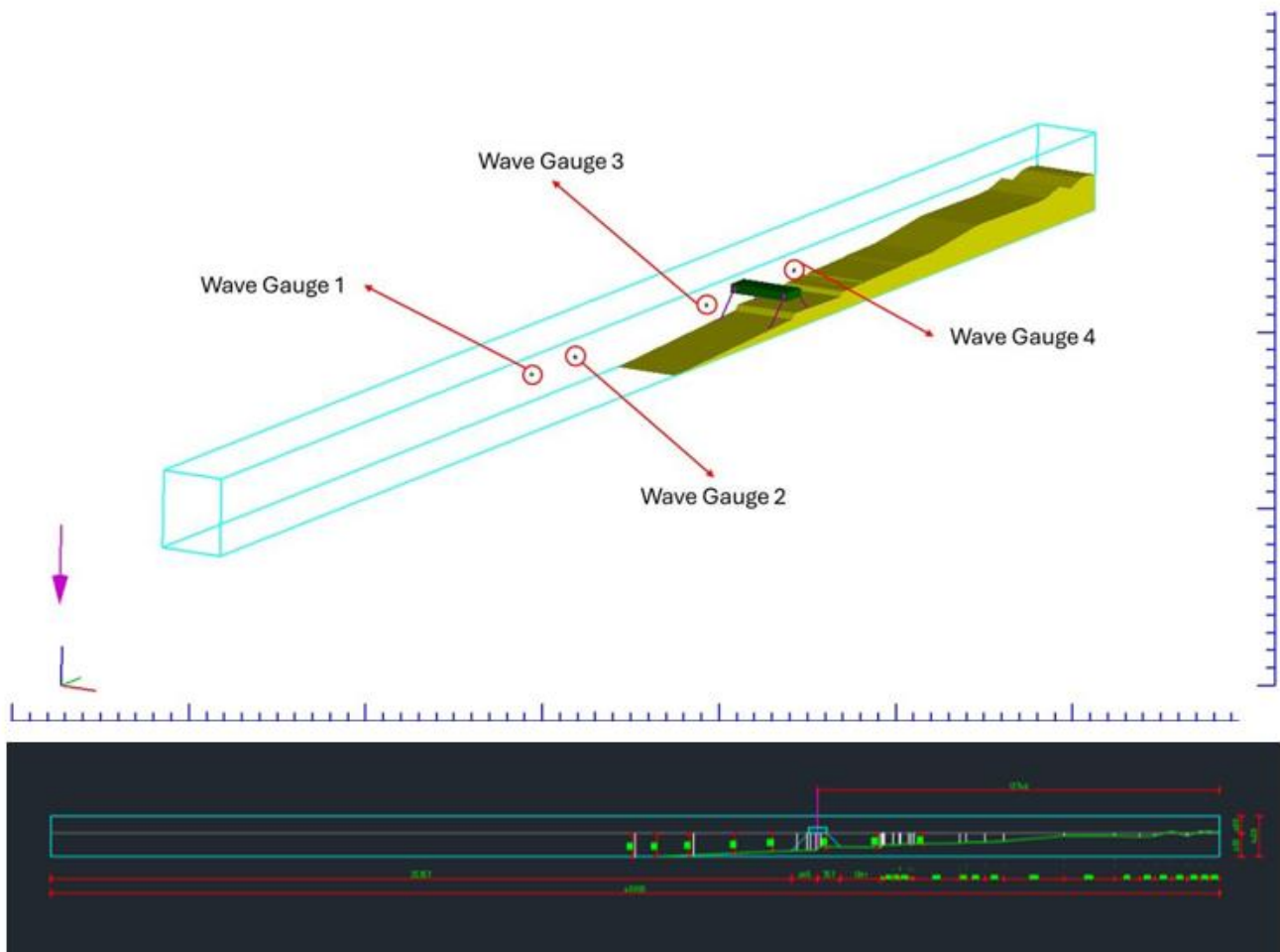


Figure 4.
Model Placement Position Floating Breakwater and Wave Probe on Wave Flume.

2.6. Equipment Calibration

In order to obtain physical modeling that matches the prototype conditions, it is necessary to carry out calibration to reduce errors during model testing. Because the function of the *wave probe* greatly influences the results of this test, the calibration process for the *wave probe* must be done. The calibration process is carried out by recording the position zero point from the *wave probe* and then recording the calibration by raising and lowering the *wave probe* 15 cm from the zero point position. After the calibration recording process is complete, the *wave probe* must be returned to its original position. This calibration is done to find the relationship between changes in the electrode immersed in water and changes in voltage recorded in the recorder.

2.7. Model Testing

Physical model testing was carried out to obtain wave transmission data for the floating *breakwater mooring system* and *pile system*. Testing was conducted on regular waves; the wave height range was determined to be between 10 and 20 cm, and the period range was between 0.5 and 3.0 seconds. Data recording takes time according to the wave period, with a minimum criterion of 10 waves [16] or 25 waves according to statistical rules; data collection takes at least 30 seconds to 1 minute. In this study, testing was carried out for 5 minutes in each model scenario. In the transmission test, the transmission coefficient K_t was calculated, which is the ratio of the transmission wave height (H_t) to the incoming wave (H_i).

3. Results and Discussion

3.1. Wave Transmission on Floating Breakwater Mooring System

The determination of the transmission coefficient of the experimental results is based on the incoming wave energy. The results of the calculation of the transmission coefficient for floating breakwater mooring types for models MU1A and MU1B at a depth of d_{50} due to *regular waves* are shown in the following Table 3. Based on the table, there is a relationship between the transmission coefficient value K_t and changes in test parameters, namely the width and draft of the model. In general, the influence of the model parameters is almost constant when viewed from the width of the model. Reviewed

from the mooring system consequence *regular waves*. It can be shown that at a depth of d50, it tends to have the smallest Kt value, namely 0.621 and 0.622, when compared to a depth of d80, namely 0.734 and 0.935.

Table 3.
Experimental Transmission Coefficient (Kt) d50 at Floating Breakwater Mooring System Consequence Regular Wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	28/11/2024	MU1A d50 L137 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.906	12.099	11.603	10.461	0.902
2	29/11/2024	MU1A d50 L137 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	9.802	11.464	11.731	9.814	0.837
3	29/11/2024	MU1A d50 L137 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.268	10.397	11.851	9.160	0.773
4	29/11/2024	MU1B d50 L137 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.096	10.860	11.823	9.536	0.807

Table 4.
Transmission Coefficient (Kt) Experimental d80 at Floating Breakwater Mooring System Consequence of Regular Wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	03/12/2024	MU1A d80 L192 draft10 f0125 A010 (Regular Wave)	10	20	0.125	8.000	21.586	19.359	23.598	30.697	1.219
2	03/12/2024	MU1A d80 L192 draft10 f0167 A010 (Regular Wave)	10	20	0.167	5.988	18.026	20.409	14.986	21.541	0.734
3	03/12/2024	MU1A d80 L192 draft10 f0250 A010 (Regular Wave)	10	20	0.250	4.000	14.680	13.600	13.784	12.730	1.014
4	02/12/2024	MU1B d80 L192 draftkosong f0125 A010 (Regular Wave)	10	20	0.125	8.000	25.409	11.597	20.234	20.589	1.745
5	03/12/2024	MU1B d80 L192 draftkosong f0125 A010 (Regular Wave)	10	20	0.125	8.000	25.837	22.386	20.933	21.791	0.935
6	03/12/2024	MU1B d80 L192 draftkosong f0167 A010 (Regular Wave)	10	20	0.167	5.988	19.681	13.199	23.612	17.506	1.789
7	03/12/2024	MU1B d80 L192 draftkosong f0250 A010 (Regular Wave)	10	20	0.250	4.000	14.687	12.111	14.929	13.816	1.233
8	02/12/2024	MU1B d80 L192 draft10 f0167 A010 (Regular Wave)	10	20	0.167	5.988	15.133	13.038	17.220	25.789	1.321
9	02/12/2024	MU1B d80 L192 draft10 f0250 A010 (Regular Wave)	10	20	0.250	4.000	15.993	12.282	15.899	13.379	1.295

The same is true if you look at the consequences of *irregular waves*. It can be shown that at a depth of d50, it tends to have the smallest Kt value, namely 0.773 and 0.807, when compared to a depth of d80, namely 0.995.

Table 5.

Experimental Transmission Coefficient (Kt) d50 at Floating Breakwater Mooring System Consequence Irregular Wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	28/11/2024	MU1A d50 L137 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.906	12.099	11.603	10.461	0.902
2	29/11/2024	MU1A d50 L137 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	9.802	11.464	11.731	9.814	0.837
3	29/11/2024	MU1A d50 L137 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.268	10.397	11.851	9.160	0.773
4	29/11/2024	MU1B d50 L137 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.096	10.860	11.823	9.536	0.807

Table 6.

Transmission Coefficient (Kt) Experimental d80 at Floating Breakwater Mooring System Consequence of Irregular Waves.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	03/12/2024	MU1A d80 L192 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	11.689	11.072	11.314	10.352	1.022
2	03/12/2024	MU1A d80 L192 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	11.343	10.482	10.434	9.679	0.995
3	03/12/2024	MU1A d80 L192 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	11.364	10.766	11.242	10.303	1.044
4	03/12/2024	MU1A d80 L192 draftkosong T6 H02 (Irregular Wave)	10	20	0.167	6.000	11.896	10.793	12.624	12.891	1.170
5	03/12/2024	MU1A d80 L192 draft10 T6 H02 (Irregular Wave)	10	20	0.167	6.000	12.088	11.196	11.869	13.104	1.060
6	03/12/2024	MU1A d80 L192 draft10 T6 H02 (Irregular Wave)	10	20	0.167	6.000	11.113	10.200	12.814	12.706	1.256

Based on this, it can be explained that the construction *floating breakwater*, if applied in the field with a mooring system, will result in *regular wave* damping, providing a smaller Kt. In this case, it is important to consider the installation location of the *floating breakwater*. If the protected area from wave impact is a port, then the effects of wave reflection and transmission should be prioritized, as wave reflection can influence ship movement. When the reflected wave's timing coincides with the wave's natural period, wave resonance may occur, potentially disrupting ship stability. When used to reduce coastal erosion or abrasion, the reflection and transmission effects should be optimized to minimize the impact of wave reflection. The goal is to achieve a balance where wave reflection is maximized and wave transmission is minimized, ensuring the *floating breakwater* provides effective coastal protection.

3.2. Wave Transmission Floating Breakwater Pile System

On the *floating breakwater pile system*, it can be seen that the empty draft MU2B d50 model provides better wave attenuation than other models (Table 7). The lowest wave transmission is obtained at the value due to a regular wave. In Tables 7 and 8, it is known that the greatest wave attenuation occurs at a transmission coefficient of 0.653.

Table 7.

Experimental Transmission Coefficient (Kt) d50 at Floating Breakwater Pile System consequence Regular Wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)			Ht Current (cm)	Kt
							WP1	WP2	WP3	WP4	
1	18/11/2024	MU2A d50 L137 draftkosong f0125 A0025 (Regular Wave)	2.5	5	0.125	8.000	1.868	0.860	3.396	3.593	1.058
2	18/11/2024	MU2A d50 L137 draftkosong f0167 A0025 (Regular Wave)	2.5	5	0.167	5.988	3.980	5.297	5.383	4.017	0.746
3	18/11/2024	MU2A d50 L137 draftkosong f0250 A0025 (Regular Wave)	2.5	5	0.250	4.000	2.625	3.137	1.881	2.905	1.545
4	18/11/2024	MU2A d50 L137 draftkosong f0125 A005 (Regular Wave)	5	10	0.125	8.000	5.128	3.089	7.322	8.102	1.106
5	18/11/2024	MU2A d50 L137 draftkosong f0167 A005 (Regular Wave)	5	10	0.167	5.988	8.102	11.107	11.368	8.362	0.736
6	18/11/2024	MU2A d50 L137 draftkosong f0250 A005 (Regular Wave)	5	10	0.250	4.000	6.279	5.925	5.023	6.661	1.326
7	18/11/2024	MU2A d50 L137 draftkosong f0167 A010 (Regular Wave)	10	20	0.167	5.988	18.839	12.614	21.573	14.098	0.653
8	18/11/2024	MU2A d50 L137 draftkosong f0250 A010 (Regular Wave)	10	20	0.250	4.000	11.391	12.625	11.682	11.919	1.020
9	18/11/2024	MU2A d50 L137 draft10 f0250 A010 (Regular Wave)	10	20	0.250	4.000	11.671	12.775	13.332	13.706	1.028
10	26/11/2024	MU2B d50 L137 draftkosong f0167 A010 (Regular Wave)	10	20	0.167	5.988	15.698	16.562	21.468	15.244	0.710
11	26/11/2024	MU2B d50 L137 draftkosong f0250 A010 (Regular Wave)	10	20	0.250	4.000	13.045	14.738	7.557	15.086	1.996
12	26/11/2024	MU2B d50 L137 draft10 f0167 A010 (Regular Wave)	10	20	0.167	5.988	15.008	16.441	21.289	14.798	0.695
13	26/11/2024	MU2B d50 L137 draft10 f0250 A010 (Regular Wave)	10	20	0.250	4.000	12.588	6.121	15.165	13.656	0.900

In theory, if the structure is a *floating breakwater*, the wider the Kt value will tend to decrease because the distance traveled by the wave is longer, so that the resulting wave reduction is also greater. Based on the data above, it is known that the effect of the relative structure width on the transmission coefficient is inversely proportional. This means that the wider the structure, the smaller the transmission coefficient will be. These results are in accordance with research conducted by Heng [13].

Table 8.

Transmission Coefficient (Kt) Experimental d80 at Floating Breakwater Pile System consequence Regular Wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	21/11/2024	MU2A d80 L192 draftkosong f0125 A010 (Regular Wave)	10	20	0.125	8.000	21.261	21.024	25.632	32.994	1.219
2	25/11/2024	MU2A d80 L192 draftkosong f0125 A010 (Regular Wave)	10	20	0.125	8.000	14.978	19.027	26.356	37.525	1.385
3	21/11/2024	MU2A d80 L192 draftkosong f0167 A010 (Regular Wave)	10	20	0.167	5.988	12.528	20.345	13.807	20.589	0.679
4	25/11/2024	MU2A d80 L192 draftkosong f0167 A010 (Regular Wave)	10	20	0.167	5.988	19.715	19.113	26.464	37.626	1.385
5	25/11/2024	MU2A d80 L192 draftkosong f0167 A010 (Regular Wave)	10	20	0.167	5.988	10.590	19.818	13.343	11.857	0.673
6	21/11/2024	MU2A d80 L192 draftkosong f0250 A010 (Regular Wave)	10	20	0.250	4.000	12.809	11.531	14.161	13.066	1.228
7	25/11/2024	MU2A d80 L192 draftkosong f0250 A010 (Regular Wave)	10	20	0.250	4.000	10.321	13.117	15.369	14.615	1.172
8	22/11/2024	MU2A d80 L192 draft10 f0125 A010 (Regular Wave)	10	20	0.125	8.000	18.095	17.019	25.026	34.550	1.470
9	22/11/2024	MU2A d80 L192 draft10 f0167 A010 (Regular Wave)	10	20	0.167	5.988	12.753	6.830	12.176	18.880	1.783
10	22/11/2024	MU2A d80 L192 draft10 f0250 A010 (Regular Wave)	10	20	0.250	4.000	12.094	10.792	13.334	13.974	1.236
11	25/11/2024	MU2B d80 L192 draft10 f0125 A010 (Regular Wave)	10	20	0.125	8.000	17.249	19.846	25.713	35.247	1.296
12	25/11/2024	MU2B d80 L192 draft10 f0167 A010 (Regular Wave)	10	20	0.167	5.988	10.411	17.578	12.912	21.010	0.735
13	25/11/2024	MU2B d80 L192 draft10 f0250 A010 (Regular Wave)	10	20	0.250	4.000	11.903	12.608	14.320	14.320	1.136

Based on experiments, it can be shown that the wider the model arrangement, the better. *Floating breakwater* waves that will be reflected will be bigger. Likewise, if we look at the effects of an irregular wave, it can be shown that at a depth of d50, it tends to have the smallest Kt value, namely 0.841 and 0.82.

Table 9.

Experimental Transmission Coefficient (Kt) d50 at Floating Breakwater Pile System consequence Irregular Wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	26/11/2024	MU2A d50 L137 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	11.609	10.735	11.945	10.234	0.857
2	21/11/2024	MU2A d50 L137 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	11.645	11.731	11.805	9.933	0.841
3	26/11/2024	MU2B d50 L137 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	11.247	10.878	11.744	10.356	0.882

Table 10.

Transmission coefficient (Kt) experimental d80 at floating breakwater pile system consequence irregular wave.

No	Date	Test Model	A input (cm)	Hi Input (cm)	f input (Hz)	T Input (s)	Hi Current (cm)		Ht Current (cm)		Kt
							WP1	WP2	WP3	WP4	
1	22/11/2024	MU2A d80 L192 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	10.438	10.164	11.223	10.328	1.104
2	22/11/2024	MU2A d80 L192 draftkosong T6 H02 (Irregular Wave)	10	20	0.167	6.000	9.744	10.509	12.433	12.151	1.183
3	22/11/2024	MU2A d80 L192 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	9.681	9.721	11.131	12.392	1.145
4	22/11/2024	MU2A d80 L192 draft10 T6 H02 (Irregular Wave)	10	20	0.167	6.000	11.140	10.742	12.943	12.332	1.205
5	25/11/2024	MU2B d80 L192 draftkosong T4 H02 (Irregular Wave)	10	20	0.250	4.000	8.954	10.086	10.979	9.545	1.089
6	25/11/2024	MU2B d80 L192 draftkosong T6 H02 (Irregular Wave)	10	20	0.167	6.000	10.182	11.374	11.602	12.745	1.020
7	25/11/2024	MU2B d80 L192 draft10 T4 H02 (Irregular Wave)	10	20	0.250	4.000	8.886	9.818	10.699	9.765	1.090
8	25/11/2024	MU2B d80 L192 draft10 T6 H02 (Irregular Wave)	10	20	0.167	6.000	9.843	11.193	11.783	13.378	1.053

4. Conclusion

In this research, the structure has been successfully designed as a floating *breakwater mooring system* and *pile system* which tested its wave transmission with experiments. Based on the experimental results, it can be concluded that the test model that can dampen the largest waves is the test model that has a smaller transmission value of 0.621 (MU1A d50 L137 draft10 f0167 A010 / Regular Wave). The results of the study indicate that the wave transmission coefficient (Kt) is greatly influenced by the size of the breakwater material, the type of wave (regular and irregular), and the model configuration used. Larger-sized materials (d80) tend to produce higher Kt values than smaller-sized materials (d50), indicating a greater ability to transmit wave energy. In addition, irregular waves provide higher Kt values than regular waves, indicating that more random or natural wave conditions allow more energy to pass through the structure. Model configurations with or without draft also affect the Kt value, where certain combinations produce more optimal damping performance. Overall, these findings indicate that the selection of material size and breakwater structure design must consider the dominant wave type to achieve maximum effectiveness in controlling wave energy.

The results of this study show that the smallest transmission coefficient is:

1. *Floating breakwater with mooring system*
2. *Floating breakwater placed at a depth of d50.*
3. *Floating breakwater draft 10 cm (submerged 10 cm / half-submerged).*
4. *Floating breakwater consequence of regular waves.*

References

- [1] M. W. Fousert, "Floating breakwater: A theoretical study of a dynamic wave attenuating system," Master Thesis Delft University of Technology, Delft, Netherland, 2006.
- [2] B. L. McCartney, "Floating breakwater design," *Journal of Waterway, Port, Coastal, and Ocean Engineering*, vol. 111, no. 2, pp. 304-318, 1985. [https://doi.org/10.1061/\(ASCE\)0733-950X\(1985\)111:2\(304](https://doi.org/10.1061/(ASCE)0733-950X(1985)111:2(304)
- [3] L. Z. Hales, "Floating breakwater: State of the art literature review," (Technical Report No. 81-1). Vickburg: Missisipi: U.S. Army Coastal Engineering Research Center, 1981.
- [4] H. Kelly, *Discussion about floating breakwaters*. Norfolk, Virginia: Naval Facilities Engineering Command, 1999.
- [5] G. Tsinker, *Marine structures engineering: Specialized applications*. New York: An International Thomson Publishing Company, 1995.
- [6] K. Murali and J. Mani, "Performance of cage floating breakwater," *Journal of Waterway, Port, Coastal, and Ocean Engineering*, vol. 123, no. 4, pp. 172-179, 1997.
- [7] J. Dai, C. M. Wang, T. Utsunomiya, and W. Duan, "Review of recent research and developments on floating breakwaters," *Ocean Engineering*, vol. 158, pp. 132-151, 2018. <https://doi.org/10.1016/j.oceaneng.2018.03.083>
- [8] K. Mangor, *Shoreline management guidelines*. Denmark: DHI Water & Environment, Hørsholm, 2004.
- [9] A. O. Ofuya, "On floating breakwaters ", Research Report No. CE-60 Kingston: Queen's University, 1968.

- [10] T. H. Priadi, "Parameterization of wave energy damping system with floating breakwater," presented at the Seminar on Hydraulics and Hydrology of Coastal Areas, PAU Engineering Science, Gadjah Mada University, Yogyakarta, 1988.
- [11] I. G. N. P. Dirgayusa and N. Yowono, "Wave transmission through horizontal pipe array wave breakwaters," *Media Teknik*, vol. 19, no. 3, pp. 37-44, 1997.
- [12] J. O. V. Walukow, "Study of wave transmission through a series of horizontal plates as a floating wave breaker," Unpublished Master's Thesis Program Pascasarjana, Universitas Gadjah Mada, Yogyakarta, 2000.
- [13] L. C. Heng, "System performance of a composite stepped-slope floating breakwater," Thesis Faculty of Civil Engineering, Universiti Teknologi Malaysia, 2006.
- [14] D. E. B. Sujantoko, W. Wardhana, and H. D. W. Armono, "Dynamic behavior analysis of porous saw floating breakwater under regular waves," presented at the The 7th International Seminars on Ocean and Coastal Engineering, Environmental and Management, 236-240. Natural Disaster, 2019.
- [15] E. B. Sujantoko Djatmiko, W. Wardhana, H. D. Armono, and M. F. Sholihin Ali, "Experimental study on the effect of mooring angle on mooring tension of porous saw type floating breakwater," presented at the The 7th International Seminars on Ocean and Coastal Engineering, Environmental and Natural Disaster Management, 2019.
- [16] S. Neelamani and R. Rajendran, "Wave interaction with T-type breakwaters," *Ocean Engineering*, vol. 29, no. 2, pp. 151-175, 2002.